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# THE STRESS INTENSITY FACTOR FOR RING SHAPED CRACK IN AN INFINITE ISOTROPIC SOLID 

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#### Abstract

The closed form expressions of stress-intensity factors and of crack shape are obtained by using Fourier transform method for a ring shaped crack in an isotropic solid.


## 1. Introduction

Civil engineers use circular pillars in the constructions of bridges. These pillars are made by iron frame and concrete with cement. The continuous use of bridges, the iron frame which is circular in shape leave the matrix (made of concrete and cement). This causes discontinuity in the medium. The shape of discontinuity is in ring shape. The height and radius of the of the pillar are large in comparison to radius or width of ring shaped discontinuity. Therefore it is considered as infinite three dimensional isotropic solid with ring shaped discontinuity whose axis coincides with $z$-axis.

Key Words : Stress Intensity Factor (S.I.F.), Crack Opening Displacement (C.O.D.), Fourier Transform (F.T.), Modified Bessels Function (MBF).
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Linear fracture mechanics has established itself as highly satisfory working tool in studying the phenomenon of brittle fracture and crack propagation in solid structures. The technique appear to be more effective when plane-strain conditions prevail.

The crack problems in shell's type solid structure or crack discontinuity in shell shape in solids poses limitation. Two major limitations arise from geometry and material behavior. The geometrical factors include the relative size of the crack with respect to radius of curvature of shells and orientation of crack. So far as material properties are concerned we take up isotropic homogeneous solid having shell-type discontinuity.
Erdogan and Ratwani [2] calculated the stresses causing fatigue and fracture of isotropic cylindrical shell containing circumferential crack by using numerical method. Erdogan [3] extended above method to orthotropic cylindrical shell having axial crack. Ma et. al [9] obtained stress-intensity factors for axial cracks in hollow isotropic cylindrical shell by using finite-element technique.
Liu et. al [8] analysed the crack closure effect on stress-intensity factors for circumferentially cracked cylindrical shell. Lal et. al [6] and Lal [7] has discussed thermoelastic problem with penny-shaped crack reducing the problem to Abel integral equation.
Jaunky et.al [5] discussed the mechanical response of laminated composite cylindrical panel in axial compression by using shell theories.

The problem in present research endeavour is of ring shaped crack having axis parallel to $z$-axis. The infinite 3-D isotropic body is now considered as cylinder of infinite radius and axis as $z$-axis. The ring shaped crack occupies the space $r=d, 0 \leq|z|<c, 0 \leq \theta \leq 2 \pi$ see figure-1.

The crack is formed by hydro-static force acting in medium and it is such that the crosssections obtained by $\theta=\alpha$ are same. It reduces the 3 -dimensional problem to 2 dimensional i.e. $r$ and $z$ only two variables. We take cross-section by $\theta=0$ and $\theta=\pi$, see figure 2a. It is being assumed that $\sigma_{\theta \theta}=0$ and the operator $\frac{\partial}{\partial \theta}$ is null operator. Thus the co-ordinates of any point will be r and $z$ when cylindrical co-ordinate system is taken.

Thus the physical problem is reduced to the following mixed-boundary value problem.

$$
\begin{gather*}
\sigma_{r r}(d, z)=-p(z), \quad 0 \leq|z|<c, \quad u_{r}(d, z)=0, \quad c \leq|z|<\infty  \tag{1}\\
\sigma_{r z}(d, z)=0, \quad 0 \leq|z|<\infty \tag{3}
\end{gather*}
$$

and all physical quantities, i.e., the components of stress and of displacement are zero as $r, z \Rightarrow \infty$.

We checked throughout that

$$
\begin{equation*}
u_{r}(d, z)>0, \quad 0 \leq|z|<c \tag{4}
\end{equation*}
$$

which means that crack really opens out and the faces of crack do not meet each other other than at crack tips, see Burniston [1].
The symmetry of geometry and of loading reduce the boundary and mixed-boundary conditions (1)-(3) to, see figure 2 b

$$
\begin{gather*}
\sigma_{r r}(d, z)=-p(z), \quad 0 \leq z<c, \quad u_{r}(d, z)=0, \quad c \leq z<\infty  \tag{5}\\
\sigma_{r z}(d, z)=0, \quad 0 \leq z<\infty . \tag{7}
\end{gather*}
$$

The plan of the paper is as follows: Section 1 introduces the problem and reduces to mixed-boundary value problem. Section-2 formulates the mixed-boundary value problem and reduces to dual integral equation. Section-3 solves the dual integral equation and reduces to Fredholm integral equation of second kind. Section-4 solves the Fredholm integral equation. Physical quantities are given in Section-5. This section takes one special case of loading, too.

## 2. Formulation and Reduction to the Dual integral equation

The equations of equilibrium, after using stress-strain equations, are reduced to fourth order partial differential equation in $u_{r}$ as :

$$
\begin{equation*}
\Delta^{2}\left(\Delta^{2}, u_{r}(r, z)\right)=0, \quad \Delta^{2}=\frac{1}{r} \frac{\partial}{\partial r} r \frac{\partial}{\partial r}+\frac{\partial^{2}}{\partial z^{2}} \tag{8}
\end{equation*}
$$

and the other displacement component $u_{z}$ is related with ur in the following manner

$$
\begin{equation*}
u_{z}(r, z)=\frac{1}{P}\left[(\lambda+2 \mu) \int\left\langle\frac{u_{r}}{r}+\frac{\partial u_{r}}{\partial r}\right\rangle d z+\int \frac{\partial u_{r}}{d z}\right] \tag{9}
\end{equation*}
$$

where $\lambda$ and $\mu$ are Lame's constants. We assume the solution of (8) as

$$
\begin{equation*}
u_{r}(r, z)=\int_{0}^{\infty} \cos (s z)\left[A(s) I_{1}(s r)+r B(s) I_{0}(s r)\right] d s \tag{10}
\end{equation*}
$$

Then

$$
\begin{equation*}
u_{z}(r, z)=-\frac{1}{P} \int_{0}^{\infty} \sin (s z)\left[Q A(s) I_{0}(s r)+B(s)\left\{(2+r s) I_{1}(s r)+\frac{(2-r s)}{r s} I_{0}(s r)\right] d s\right. \tag{11}
\end{equation*}
$$

Where $P=\lambda+\mu, Q=1+\mu+P$ and $I_{0}(s r), I_{1}(s r)$ are modified Bessel's functions of kind first with order zero and one. The use of stress-strain relations and (10)-(11) we get

$$
\begin{align*}
\sigma_{r r}(r, z)= & \int_{0}^{\infty} s \cos (s z)\left[-A(s)\left\{I_{0}(r s) \alpha_{0}=\frac{I_{1}(s r)}{s r}\right\}\right. \\
& \left.+B(s)\left\{\alpha_{1} I_{0}(s r)+(s r(1+Q)+1) I_{1}(s r)\right\}\right] d s  \tag{12}\\
\sigma_{r z}(r, z)= & -\frac{\mu}{P} \int_{0}^{\infty} s \sin (s z)\left[(P+Q) A(s) I_{1}(s r)+\frac{B(s)}{s}\right. \\
& \left.\left\{I_{0}(r s)\left(2 r^{2} s p+r^{2} s^{2}\right)+(r s+2) I_{1}(s r)\right\}\right] d s \tag{13}
\end{align*}
$$

where $A(s)$ and $B(s)$ are two arbitrary constants to be determined. The quantities in (10)-(13) vanish as $z$ or $|r| \rightarrow \infty$. The boundary condition (7), with (13), gives

$$
\begin{equation*}
b_{1} A(s)=-\frac{b_{2} B(s)}{2} \tag{14}
\end{equation*}
$$

with

$$
\begin{equation*}
b_{1}=I_{0}(d s)\left[2 d^{2}+\frac{d}{s}\right]+\frac{s+2}{s} I_{1}(d s), \quad b_{1}=(p+Q) I_{1}(d s) . \tag{15}
\end{equation*}
$$

Now, the substitution of $u_{r}$ and $\sigma_{r r}$ from (10) and (12), respectively, and using (14) (15), gives

$$
\begin{gather*}
\int_{0}^{\infty} \psi(s) \cos (s z) d z=0, \quad c \leq z<\infty  \tag{16}\\
\int_{0}^{\infty} s(\psi) \cos (s z) d s=-P_{1}(z), \quad 0 \leq z<c  \tag{17}\\
b_{1} \psi(s)=B(s)\left[d b_{1} I_{0}(d s)-b_{2} I_{1}(s d)\right]  \tag{18}\\
P_{1}(z)=p(z)+\int_{0}^{\infty} s \cos (s z) \psi(s) M(d s) d s  \tag{19}\\
M(s d)=\left(b_{2} b_{4}-b_{1} b_{3}-b_{5}\right) / b_{3}, \quad b_{3}=(3-Q) I_{0}(s d)+I_{1}(s d)[1+s(1+Q)]  \tag{20}\\
b_{4}=I_{0}(s d)\left[\frac{Q-P}{P}+\frac{I_{1}(s d)}{s}\right], \quad b_{5}=b_{1} d I_{0}(s d)-b_{2} I_{1}(s d) . \tag{21}
\end{gather*}
$$

Thus the problem is reduced to dual integral equation (16) - (17).

## 3. Solution of Integral Equation and Expansion of Some Function Solution of Integral Equation

The solution of dual integral equation (16) - (17) is obtained through the method of Srivastava and Lowengrub [10].
The solution is assumed as,

$$
\begin{equation*}
\pi \psi(s)=2 \int_{0}^{c} g(t) \frac{\sin s t}{t} d t \tag{22}
\end{equation*}
$$

with no loss of generality as $g(0)=0$. The use of integral

$$
\int_{0}^{\infty} \frac{\sin s t \cos s t}{t} d t= \begin{cases}\pi / 2, & s>x \\ \pi / 4, & s=x \\ 0, s<x & \end{cases}
$$

will satisfy (16) through (22).
Then using (22) in (17) and the value of integral

$$
\int_{0}^{\infty} \frac{\sin s t \sin x t}{s} d s=\frac{1}{2} \log \left|\frac{t+x}{t-x}\right|
$$

will give

$$
\begin{gather*}
g(t)=-\frac{2 t}{\pi^{2} \sqrt{c^{2}-t^{2}}}\left[\Delta_{0}(t)+\int_{0}^{c} g(\alpha) M_{1}(\alpha, t) d x\right], \quad 0 \leq t<c  \tag{23}\\
M_{1}(\alpha, t)=\int_{0}^{c} \frac{\sqrt{c^{2}-z^{2}}}{z^{2}-t^{2}} K_{1}(\alpha, z) d z, \quad \Delta_{0}(t)=\int_{0}^{c} \frac{\sqrt{c^{2}-z^{2}}}{z^{2}-t^{2}} p(z) d z  \tag{24}\\
K_{1}(\alpha, z)=\int_{0}^{\infty} M(s d) \cos (s z) \sin (s \alpha) d s \tag{26}
\end{gather*}
$$

$M(s d)$ is define in (20).

## 4. Expansion of Some Functions

We make use of expansion of modified Bessel's function $I_{v}(z)$ of order $v$. In this case $v=0$ and $v=1$.

$$
\begin{equation*}
I_{v}(z)=e^{-z} \sum_{m=0}^{d-i}(v, m)(-1)^{m}\left(\frac{z}{2}\right)^{m}, \quad(v, m)=\sqrt{v+m+\frac{1}{2}} / m!\sqrt{v+\frac{m}{2}} \tag{27}
\end{equation*}
$$

see [4]. It is real part of $I_{v}(z)$.

To get the approximate expansion of $M(s d)$ it is needed

$$
b_{2} b_{4}-b_{1} b_{3}=e^{-s d} \sum_{m=0}^{n-1} \sum_{r=0}^{n-1}(-1)^{m+r}\left(\frac{s d}{2}\right)^{m+r} e_{1}(m, r, d)
$$

with

$$
\begin{gather*}
\theta_{1}(m, r, s)=\sqrt{m+\frac{1}{2}} \sqrt{r+\frac{1}{2} e_{11}}(m, r, s) \\
e_{11}(m, r, s)=\left(r+\frac{1}{2}\right) d_{4}+d_{5}\left(r+\frac{1}{2}\right)\left(m+\frac{1}{2}\right) d_{6} \\
b_{5}=\sum_{r=0}^{n-1} \sum_{m=0}^{n-1}(-1)^{m+r}\left(\frac{s d}{2}\right)^{m+r}\left[d_{7} \sqrt{m+\frac{1}{2}} \sqrt{r+\frac{3}{2}}+d_{6} \sqrt{m+\frac{3}{2}} \sqrt{r+\frac{3}{2}}\right] \\
b_{5}^{-1}=\frac{\pi}{4} \sum_{e=0}^{\infty}\left(2 d_{7}-d_{8}\right)^{-1}\left[\frac{s d}{4}\left(\frac{2 d_{7}-3 d_{8}}{2 d_{7}-d_{8}}\right)^{e}\right] \\
M(s d)=\pi \sum_{R=0}^{\infty} \sum_{l=0}^{\infty} \sum_{p=0}^{l} \sum_{m=0}^{n-1} \sum_{r=0}^{n-1}(-1)^{m+r+p+l} e_{2}(m, r, d) \\
\left(\frac{\alpha d}{2}\right)^{m+r+l-2 p-2 k}{ }^{m} C_{p}\binom{p}{k}\left(d_{1} s-2\right)^{k} \tag{28}
\end{gather*}
$$

where $d_{1} \sim d_{8}$ alongwith other variables are given in appendix-I.

## 4. SOolution of Fredholm Integral Equation

To solve Fredholm integral equation given in (23), we expand the function $g(t)$ in terms of ' $d$ ' i.e. distance of ring shaped crack from $z$-axis.

$$
\begin{equation*}
g(t)=\sum_{r=0}^{\infty} g_{r}(t) d^{-r} \tag{29}
\end{equation*}
$$

and then substitute (29) in (23) and compare the coefficients of $\left\{d^{-m}\right\}$ from both sides. Before we proceed for above analysis we take appropriate values of $k, l, p, m, r$ so that in the expansion of $M(s d)$ we retain upto $d^{-5}$ only. Then from (28)
$M(s d)=\frac{2}{3 P}\left[\frac{t_{6}}{d^{2} s}+\frac{1}{d^{4} s^{2}}\left\langle t_{1}+\frac{2 \sqrt{\pi}}{3}\right\rangle+\frac{1}{d^{6} s^{3}}\left\langle\frac{2 \sqrt{p}}{3} t_{7}+\frac{4 \pi}{9} t_{6}-t_{2}\right\rangle\right]\left[1+\frac{P+Q}{2 P d}+\frac{\sqrt{\pi}}{P d^{2}}\right]$.

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This $M(s d)$ gives $K_{1}(\alpha, z)$ from (26), after evaluating integrals, as

$$
\begin{gather*}
K_{1}(\alpha, t)=\frac{1}{d^{2}}\left[t_{8}+\frac{t_{0}}{d}+\frac{t_{10}}{d^{3}}\right] T(\alpha, t)  \tag{30}\\
T(\alpha, t)=\frac{1}{2}\left[\alpha \log \left|\frac{\alpha+t}{\alpha-t}\right|+t^{2} \log \left|\alpha^{2}-t^{2}\right|-|\alpha-t|\right], \quad 0 \leq \alpha, \quad t<c \tag{31}
\end{gather*}
$$

where $t_{i}, i=1,2, \cdots, 10$ are given in Appendix-II. Evaluate $M_{1}(\alpha, t)$ from (24) after using (30) and evaluating integrals which is given as,

$$
\begin{gather*}
M_{1}(\alpha, t)=\left(\frac{t^{8}}{d^{2}}+\frac{t_{9}}{d^{3}}+\frac{t_{10}}{d^{5}}\right) T(\alpha, t)  \tag{32}\\
T(\alpha, t)=\frac{\pi}{2}\left(\alpha^{2}-3 t^{2}\right) \alpha^{2}+\frac{\pi \alpha}{2} I_{2}(t) \\
I_{2}(t)=-c+\frac{\sqrt{c^{2}-t^{2}}}{2} I_{21}(t), I_{21}(t)=\log \left|\frac{c+\sqrt{c^{2}+t^{2}}}{c-\sqrt{c^{2}-t^{2}}}\right| . \tag{33}
\end{gather*}
$$

Now we use (29) in (23) and relevant function there in and compare coefficients of $\left\{d^{-m}\right\}, m=0,1,2,3,4,5$ only. There

$$
\begin{aligned}
g_{0}(t) & =-\frac{2}{\pi^{2}} \psi_{0}(t) H_{0}(t), g_{1}(t)=0, \psi_{0}(t)=\frac{t}{\sqrt{c^{2}-t^{2}}}, \quad 0 \leq t<c \\
g_{2}(t) & =\frac{4 t_{8}}{\pi} \psi_{0}(t)\left[-d_{0} I_{2}(t)+\frac{\pi}{12}\left\langle T_{5}(t)-3 t^{2} I_{3}(t)-\frac{\pi^{2}}{4}\right\rangle\right] \\
d_{0} & =\int_{0}^{\infty} H_{0}(t) I_{21}(t) d t \\
T_{2 n+1}(t) & =\int_{0}^{\infty} \frac{\alpha^{2 n+1}}{\sqrt{c^{2}-\alpha^{2}}\left(\alpha^{2}-t^{2}\right)}=-P_{2 n-1}+t^{2} T_{2 n-1}(t) \\
P_{2 n+1} & =\frac{(n!)^{2} c^{2 n+1}}{2^{2 n+2}(4 n+1)!}, \quad T_{1}(t)=\frac{2}{\sqrt{c^{2}+t^{2}}} I_{21}(t) \\
g_{3}(t) & =0 \\
g_{4}(t) & =\frac{2}{3 \pi^{2}} \psi_{0}(t) t_{4}^{2}\left[q_{14}-t^{2} q_{15}-q_{16} I_{2}(t)\right] \\
g_{5}(t) & =-\frac{34 t_{8}^{2}}{\pi^{2}} \psi_{0}(t)\left[q_{17}+t^{2} q_{18}-I_{21}(t) I_{22}(t)\right] \\
I_{22}(t) & =d_{0} t_{10} \sqrt{c^{2}-t^{2}}+\frac{5 \pi t^{4}}{6 \sqrt{c^{2}-t^{2}}}+t_{9} q_{1} b
\end{aligned}
$$

where $q_{i}, i=0,1,2, \cdots, 25$ are given.

Appendix III. These are constants depending upon elastic properties and geometrical parameters $d$ and $c$. Thus substituting $g_{i}(t), i=0,1,2, \cdots, 5$ in (29) from above

$$
\begin{gather*}
g(t)=-\frac{2}{\pi^{2}} \psi_{0}(t)\left[H_{0}(t)+\frac{H_{1}(t)}{d^{2}}\left\langle q_{23}+\frac{t^{4} q_{24}}{d^{2}}+\frac{1}{a^{3}}\left\{I_{21}(t)+I_{29}(t)\right\}\right\rangle\right]  \tag{34}\\
I_{23}(t)=6 q_{22} c^{2}-6 t^{2} q_{3}+t^{4} q_{25}+\frac{t_{8}}{d} I_{2}(t) \tag{35}
\end{gather*}
$$

## 5. Physical Quantities in General and a Special Loading

The crack opening displacement and normal stress-component are quantities which are important in fracture design parameters.

## Crack Opening Displacement

The crack opening displacement is the value of integral in (16) for $z$ in $[0, c)$. Now using (22) in (16) and evaluating the integral we get

$$
\begin{equation*}
u_{r}(d, z)=\int_{z}^{c} g(t) d t, \quad 0 \leq z<c \tag{36}
\end{equation*}
$$

where $g(t)$ is to be taken from (34).

## Stress Components

The component of shear stress at $r=d$ is assumed to be zero for all $z$.

## Normal Stress

The normal stress component is obtained from (17) for $z$ in $(c, \infty)$ after taking second term on left hand side and is given as

$$
\begin{align*}
\sigma_{r r}(d, z)=\frac{1}{\pi} \psi_{0}(z) & {\left[H_{0}(z)+\frac{1}{d^{2}}\left\langle q_{23}+\frac{z^{4} q_{24}}{d^{2}}+\frac{1}{d^{3}}\left\{I_{21}(z)+I_{23}(z)\right\}\right\rangle\right] } \\
& -\int_{0}^{c} g(p) m_{2}(p, z) d p, c<z<\infty \tag{37}
\end{align*}
$$

$\left.m_{2}(p, z)=\int_{0}^{\infty} M(d s)\right] \cos (s z) \sin (d s) d s$.
It possesses Cauchy type singularly at crack tip $(d, c)$.

## Stress Intensity Factor

The stress-intensity factors at crack tips are defined as

$$
\begin{equation*}
\left(K_{c}, N_{c}\right)=\lim _{c \rightarrow e^{-}} \sqrt{z-c}\left(\sigma_{r r}(d, z), \sigma_{r z}(d, z)\right) \tag{38}
\end{equation*}
$$

But $N_{c}=0$. Using (37) in (38) and evaluating the limit $K_{c}$ is given as:

$$
\begin{gather*}
K_{c}=\frac{1}{\pi} \sqrt{\frac{c}{2}}\left[H_{0}(c)+\frac{1}{d^{2}} H_{1}(c)\right],  \tag{39}\\
H 1(c)=q_{23}+\frac{c^{4} q_{24}}{d^{2}}+\frac{1}{d^{3}}\left\{I_{21}(c)+I_{23}(c)\right\} .
\end{gather*}
$$

## Special Case of Loading

We consider that crack was opened by constant and uniform force at crack faces, therefore,

$$
\begin{equation*}
p(z)=p_{0}=\text { constant } \tag{40}
\end{equation*}
$$

Thus

$$
\begin{equation*}
H+_{0}(t)=p_{0} \frac{\pi}{2}=\text { constant. } \tag{41}
\end{equation*}
$$

Substituting (41) in (34) and evaluating integrals

$$
\begin{align*}
& u_{r}(d, z)=\frac{P_{0}}{\pi} \sqrt{c^{2}-z^{2}}\left[1+\frac{1}{d^{2}}\left\{\left.q_{23}+\frac{q_{24} c^{4}}{d^{2}}+\frac{1}{d^{3}}\left\langle\frac{\sqrt{c^{2}-z^{2}}}{c} \log \right| \frac{c+\sqrt{c^{2}-z^{2}}}{c-\sqrt{c^{3} 2-z^{2}}} \right\rvert\,\right.\right. \\
& \left.\left.\left.-2\left(c-\sqrt{c^{2}-z^{2}}+\left(3 c^{2} q_{22}+3 c^{2} q_{23}\right)+\left(1-\frac{1 c^{2}-z^{2}}{3 c^{2}}\right)\right)\right\rangle\right\}\right], 0 \leq z<c \tag{42}
\end{align*}
$$

Thus, he closed form expressions for crack opening displacement and of stress-intensity factor $k_{c}$ are obtained and given by (42) and (39), respectively.



$$
u_{r}(d, z)=0, c \leq z<\infty
$$

$p(z)=p_{0}:$ Constant and uniform
$\uparrow \begin{gathered}8 \\ \uparrow \\ N\end{gathered}$
force at crack faces


FIGURE- 3. THE CRACKS OPENING DUE TO CONSTANT AND UNIFORM FORCE AT CRACK FACES

## Appendix-I

$d_{1}=d^{2}(2 p+s), d_{2}=\frac{s+2}{s}, d_{3}=14 s(1+Q), \quad d_{4}=\frac{d_{1}}{s}+\frac{Q-P}{P} d_{2}-(P+Q)(3-Q)$

$$
\begin{gathered}
d_{5}=\frac{d_{1}(Q-P)}{P}, d_{6}=\frac{d_{2}}{s}-(P+Q) d_{3}, \quad d_{7}=P+Q+d_{1}, d_{8}=d_{2} \\
e_{2}(m, r, s)=e_{21}(m, r, s) \sqrt{m+\frac{1}{2}} \sqrt{r+\frac{1}{2}} \\
e_{21}(m, r, s)=e_{11}(m, r, s)-\sqrt{r+\frac{1}{2}}\left(d_{7}\right)+d_{8}\left(m+\frac{1}{2}\right)\left(r+\frac{1}{2}\right) .
\end{gathered}
$$

## Appendix - II

$t_{1}=[2(1+Q) \sqrt{\pi}-3] / 4, \quad 2 t_{2}=\sqrt{\pi}-6 Q, t_{3}=t_{2}, t_{4}=2(P+\sqrt{\pi}-2 / P)$, $t_{5}=(4 Q-7 P) / 4 P, \quad t_{6}=t_{4} t_{5}+3(1+Q) P$, $t_{7}=\sqrt{\pi}\left(t_{4}+8 t_{5}\right) / 2-t_{1}, t_{8}=2\left[t_{7}+2 \sqrt{\pi} / 3\right] / 3 P, t_{9}=(P+Q) t_{10} / \sqrt{\pi}, \quad t_{10}=\sqrt{\pi} t_{8} / p$.

## Appendix - III

$\left(q_{0}, q_{1}\right)=\int_{0}^{c} \psi_{0}(\alpha) \alpha^{2} I O_{2}(\alpha)\left(\alpha^{2}, 1\right) d \alpha, \quad\left(q_{2}, q_{3}\right)=\int_{0}^{c} \psi_{0}(\alpha) \alpha^{2} T_{5}(\alpha)\left(\alpha^{2}, 1\right) d \alpha$ $\left(q_{4}, q_{5}\right)=\int_{0}^{c} \psi_{0}(\alpha) T_{3}(\alpha) \alpha^{4}\left(\alpha^{2}, 1\right) d \alpha, \quad q_{6}=\pi^{4}\left(P_{5}-3 e^{2} P_{1}\right) / 576, \quad q_{10}=\frac{\pi c^{2}}{4}$
$\left(q_{7}, q_{8}, q_{9}\right)=\int_{0}^{c} \alpha \psi_{0}(\alpha)\left(I_{0}(\alpha), \mathrm{E}^{2} t_{3}(\alpha)\right) d \alpha$
$q_{14}=\frac{\pi q_{0} d_{0}}{12}+\frac{\pi^{2}}{144}\left(q_{2}-3 q_{4}\right)-\frac{\pi^{4}}{576} P_{5}+\frac{\pi}{2}\left[d_{0} q_{7}+\frac{\pi}{12} q_{8}-\frac{\pi}{4} q_{9}-\frac{\pi}{96} P_{3}\right]$
$q_{15}=\frac{\psi}{4}\left[d_{0} q_{1}-\frac{\pi}{12} q_{3}-\frac{\pi}{4} q_{5}-\frac{\pi}{48} P_{3}\right], \quad q_{16}=d_{0} q_{11}+\frac{\pi}{2}\left[q_{12}-\frac{\pi}{6} q_{13}-\frac{\pi^{2}}{24} q_{10}\right]$
$q_{17}=t_{9}\left(q_{14}+c q_{16}\right)-\left(c d_{0}+\frac{\pi^{2}}{4}-\frac{\pi}{12} P_{3}\right) t_{10}, \quad q_{18}=\frac{\pi}{12}\left(1-\frac{\pi}{2}\right) P_{1}-t_{9} t_{15}$
$q_{19}=-\left[2 d_{0} c t_{8}+\frac{\pi^{3}}{48} t_{8}-\frac{\pi}{12} P_{3}\right], \quad q_{20}=\frac{\pi}{12} t_{8}\left[q_{0} d_{0}+\frac{\pi}{2}\left(q_{2}-3 q_{3}\right)\right]$
$q_{21}=-\frac{\pi}{3} P_{1}, \quad q_{22}=-2 d_{0} t_{0}, \quad q_{23}=q_{19}+\frac{q_{20}}{d^{2}}-\frac{\pi t_{8} q_{1}}{d^{3}}, \quad I_{0}(x)=\left(c^{2}-2 x^{2}\right) \frac{\pi}{2}$
$q_{24}=q_{21}-\frac{\pi q_{1} d_{0} d_{8}}{12 d^{2}}-\frac{8 t_{8} q_{1}}{d^{3}}, \quad q_{25}=(6+\pi) t_{8}$.

## References

[1] Burniston E. E., An example of partially closed Griffith-crack, Int. J. Fracture Mech. (1969), 17-24.
[2] Erdogan F. and Ratwani M. (1970), Fatigue and fracture of cylindrical shells containing a circumferential crack., Int. J. Fracture Mech., 6 (1970), 371-392.
[3] Erdogan F., An axial crack in specially orthotropic cylindrical shell. Mechanics of fracture of plates and shells. Edited by G. C. Ashih Nocrdoff International Publishing London, (1977).
[4] Gradstyen I. S. and Rizhik I. N., Tables of Integrals, Series and Products, Academic press London, (1965).
[5] Jaunky N. and Knight Jr. N. F., An assessment of shell theories for buckling of circular cylindrical laminated composite planes loaded in axial compression. Int. J. Solid Struct., 36(3) (1999), 799-820.
[6] Lal M. and Pandey D., The axis symmetric thermo-elastic stress distribution in an infinite solid containing penny-shaped crack. Proc. Nat. Acad. Sci., 45(a) (1975), 175-180.
[7] Lal M., Some axis symmetric thermal stress distribution in an elastic solid containing an annular crack. Lett. Apl. Egng. Sci., 20(11) (1982), 1261-1273.
[8] Liu R. Z., Tie W. X. J. and Wang C. H., Crack closure effect on stress- intensity factors of an axially and circumferenteally cracked cylindrical shell. Int. J. Fraction, 125(3-4) (2004), 227-248.
[9] Ma C. C., Huarg J. I. and Tsai C. H., Weight functions and stress-intensity factors for axial cracks in hollow cylinder's, J. Pressure Vessel Tech. Nov., 116 (1994), 423-430.
[10] Srivastava K. N. and Lowengrub M.,Finite Hilbert transform technique for triple integral equation with trigonometric Kernels. Proc. Roy. Soc. Edinb., A68 (1970), 309-321.

